

Memorandum

Date:

March 24, 1998

From:

.M. Allen/N. Dbaibo SC Geotechnical Branch

Phone:

(360) 709-5469

Subject:

t: SR- 167, OL-2305

15th Avenue SW to 15th Avenue NW

HOV Lanes - Stage 3, MP 13.73 to MP 15.76

Geotechnical Recommendations for

Walls No. 3 and 10 Design

and Review of Walls 1, 2, 4, 5 and 6

To: A.E. Stiles / N.J. Creelman NW Region, NB82-29

As requested, we are providing you with geotechnical recommendations for the design of retaining walls No. 3 and 10 needed to construct the AL2 line on-ramp from SR-516 to SR-167 and the AL1 on-ramp from S. 277 Street to SR-167 as part of the the 15th Avenue SW to 15th Avenue NW HOV lane on SR-167. Based on the site plan provided to our office, wall 3 begins at station Al2 100+00 and ends at station Al2 101+40.72. The maximum exposed wall height will be about 1.5 m (5 ft). Wall 10 begins at Al1 11+50 and ends at station AL1 12+75 with a maximum exposed height of about 1.12 m (3.68 ft).

In addition we have reviewed the geotechnical recommendations for the design of walls 1, 2, 4, 5 and 6 presented in WSDOT Geotechnical Report dated January 18, 1994.

It should be noted that the wall numbering has changed since the completion of the 1994 geotechnical report. A summary table of wall designation and stationing during stage 1, 2 and 3 was provided by the NW Region Office and is enclosed for reference.

The analysis, conclusions and recommendations contained in this memorandum are based on the project description, site conditions as they existed at the time of our site visit and preparation of this memo and subsurface information supplied your office and in our files. It is further assumed that the subsurface conditions as interpreted from the borings are representative of the subsurface conditions throughout the project area. If during construction, subsurface conditions are different from those encountered in the exploratory borings, or appear to be present beneath or beyond the excavations, we should be advised so that we can assist you and re-evaluate our recommendations.

Site Soil Conditions

Wall No. 3

The soil conditions at the location of wall 3 were interpreted from existing exploratory borings performed in the vicinity of the proposed wall alignment. The existing borings, drilled by WSDOT, dated August 24, 1982 and September 22, 1993, consisted of four borings extending to a depth of 7.6 m (25 ft). The borings were originally performed for the traffic signals along SR-167. Additional borings in the vicinity were performed by Terra Associates in 1991 for SR-516 undercrossing. These borings are numbered B-7 and B-12 in the referenced report. The borings generally indicate that the site is underlain by brown, loose sandy silt to a depth of approximately 3.65 m (12 ft). Below this sandy silt is black, loose to medium dense, fine to medium sand was encountered.

Groundwater was encountered at a depth of approximately 3.65 m (12 ft) below existing grades.

Wall No. 10

The soil conditions at the location of wall no. 10 were inferred from the borings along the northbound on-ramp to SR-167. These borings, H-15-93 and H-16-93 were drilled as part of the 1994 WSDOT investigation. These borings indicate that the subsurface soil conditions consist of dense and very dense sandy gravel to a depth ranging between 2.43 m (8 ft) and 4.26 m (14 ft) below existing grades. Underlying this dense gravel is soft and medium stiff grey silt and sandy silt.

Groundwater was encountered at a depth of approximately 3.05 m (10 ft) below existing grades.

Geotechnical Recommendations

We have evaluated several wall options, based on settlement considerations, it appears that the most economical wall, that could accommodate settlement, would be a gabion wall (Figure 1) or a concrete modular block wall, such as UltraBlock System (Figure 2). We performed wall stability analyses for the types of walls, including sliding and overturning at each individual basket and block elevation. The height of these walls were limited to three baskets or three blocks for each type of walls. The results of the stability analyses indicate that the walls meet the required criteria for sliding and overturning

If a gabion wall is selected, the basket construction and backfill should be in accordance with Standard Plans Sheet D-6. The gabion wall should be constructed in accordance with the 1998 Standard Specifications Section 6-09. In addition the following recommendations should be included in the Special Provisions for the wall design and construction:

- 1. The wall shall be placed on a level foundation in the horizontal direction perpendicular to the wall face or it could be battered at 1V:6V. The leveling pad should be composed of select borrow, per Section 9-03.14(2).
- 2. Wall face batter should be 6V:1H.
- 3. Wall base embedment should be at least 0.3 m (1 ft) below final finished grade.
- 4. The wall height should be limited to two baskets high. If in localized areas, more than two baskets are needed, then the first row of baskets should be two baskets wide.

As an alternative to a gabion basket wall, concrete modular blocks can be used to construct the proposed walls. The interlocking concrete wall systems are composed of concrete blocks measuring approximately $1.5 \times 0.75 \times 0.75 = 0.75 \times 0.75$

- 1. Blocks shall be stacked no more than two blocks in height with their short dimension perpendicular to the slope. If more than two blocks are needed, then the first block should be stacked with its longest dimension perpendicular to the face of the slope.
- 2. Blocks shall be stacked in a manner such that adjacent lower blocks are interlocked by the top blocks.
- 3. Wall face batter shall be no steeper than 1H:6V.
- 4. No reject or returned concrete shall be used in the casting of the blocks.

The base blocks should be supported on at least one foot of select borrow, per section 9-03.14(2). Drainage provisions should also be provided behind the wall system. Gravel backfill for walls, per Section 9-03.12, should be used for wall backfill. A geotextile layer should be placed between the native silty soil and the gravel backfill. Construction Geotextile for Underground Drainage, Low Survivability, Class C should be used. Special Provisions for the concrete modular retaining wall are enclosed.

Review of Geotechnical Recommendations for Walls 1, 2, 4, 5 and 6

The design recommendations for walls 1, 2, 4, 5 and 6 were addressed in a geotechnical report by WSDOT dated January 18, 1994. This report recommended that walls 1, 2, 4, 5 and 6 be constructed as mechanically stabilized earth walls (MSE). Design recommendations including soil unit weights, friction angles and allowable bearing capacities are presented in the 1994 report and are considered adequate. No change in the design recommendations is anticipated for these walls.

Closure

We trust this information is sufficient at this time. Should you have questions or require further information, please contact Nabil Dbaibo at (360) 709-5469 or Jim Cuthbertson at (360) 709-5452.

TMA:ntd

NTD

Attachments

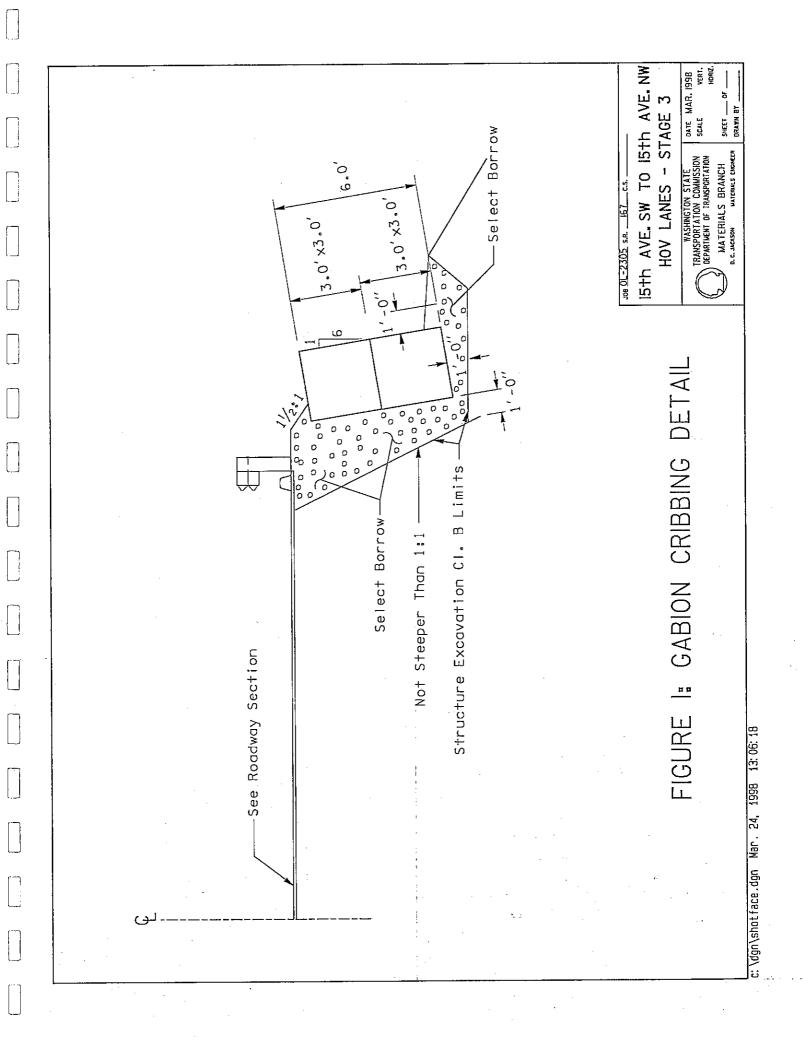
cc: M. Lwin, Bridges & Structures, 47340

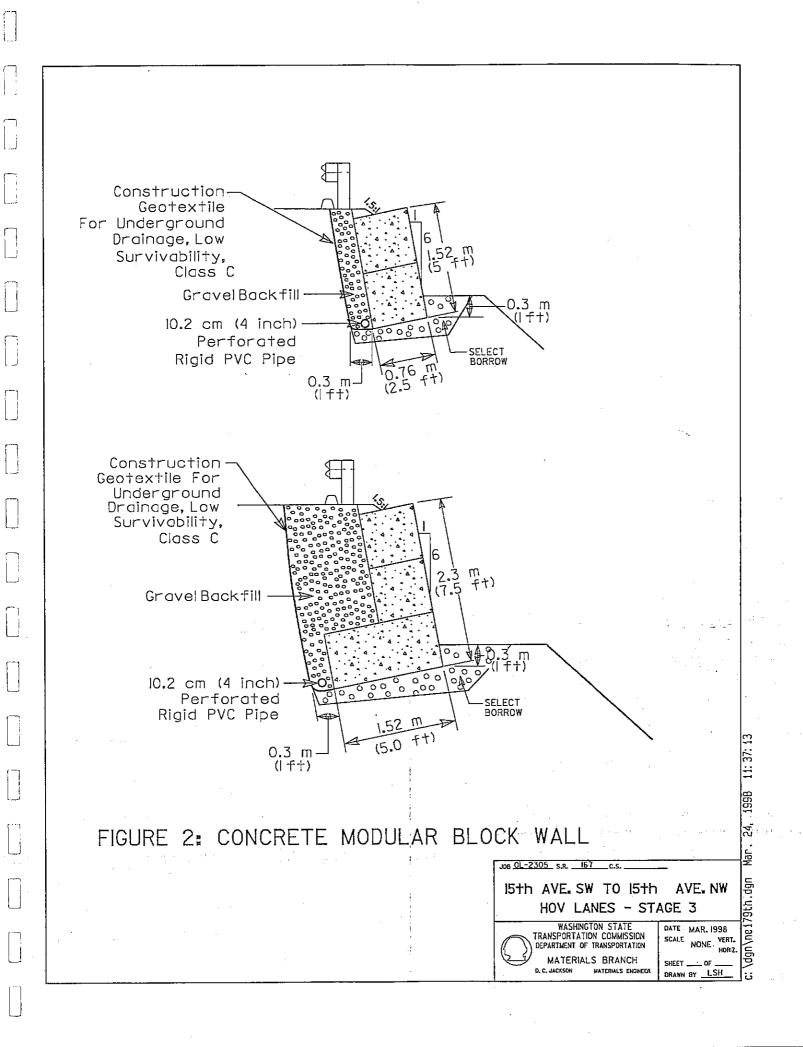
A. Young, Bridges & Structures, 47340

A. Korynta, OSC Construction, 47354

B. Rennie, NW NB82-114

J. Johnson, NW NB82-143





			- 15TH SW			<u> </u>	
		HOV L	ANES - STAGE	3			
					- 1		
isted belo	w are wa	alls to be	e designed or cons	tructed under this	project.		
	<u>_</u>			•	Old	•	Stage :
Vall Nam	<u> </u>		Begin Sta.	End Sta.	Wall #		Wall#
Vali Ivaiii							
5TH NW	A12 (S/F	3 00)	Eliminated	Eliminated	-		
51111111	ALZ (OIL		Liminated				
	· · · · · · · · · · · · · · · · · · ·		<u> </u>	<u> </u>			
	·]		DD4140.70 (27 60' BT)	LM 725+14.69 (84.71' RT)	1		. 1
o. 277th D	R1 (N/B o	<u>n)</u>	DR1' 19+70 (27.60' RT)	LW 7257 14.00 (04.71 117)			
		<u> </u>	<u> </u>	114 725:00			`
	Soils Repo		DR1' 19+80	LM 725+00 · ·			
F	S&E Shel	f Copy	DR1' 19+70 (28.6' RT)	LM 729+81 (76.37' RT)			
(stage 2)	<u> </u>			<u> </u>		 -
				<u> </u>	<u> </u>	<u>'</u>	1
So. 277th A	L1 (S/B o	n)	Eliminated	Eliminated	ļ <u> </u>	· -	<u> </u>
			·			<u> </u>	-
SR 516 AL	2' (S/B on))	LM 791+70.00 (95.15' LT	AL2' 93+40.16 (34.97' LT)	2		2
					<u> </u>	<u> </u>	
	Soils Repo	rt Sta.	LM 790+50	AL2 95+50	<u> </u>		
	PS&E She		L 790+57.50 (95.0' LT)	AL2' 94+94.90 (45.47' LT)		<u> </u>	
	(stage 2)	<u> </u>					
	(Stage Z)	!				Ţ	
OD 540 A1	at (CIP on		AL 2 100+00 0 (40.88' LT) AL2 101+40.72 (39.68' LT	None		3
SR 516 AL			None	None			
	Soils Repo	ort Sta.	Mone .	17070			
· -					 	1	-
		<u> </u>	00 PT	1 942 107 94 (00 0' PT)	3	 	4
SR 516 DF	2' (N/B or	1)	DR2' 7+94.75 (29 KT)	L 812+07.84 (90.0' RT)		 	
		<u> </u>					
	Soils Rep		DR2 8+78	L 811+08			_
	PS&E She	elf Copy_	DR2' 7+94.75 (30.27' R	T) L 812+07.84 (90.0' RT)			
	(stage 2)					-	
NOTES:	-					1 21 2 1	
Soils Re	port:	Soils re	port was compiled in I	March of 1992 under th	e title "15	th St S.	VV. 10
[<u> </u>	I South C	Grady Way"			<u>. </u>	
Stage 1:	<u> </u>	Stage 1	of SR 167 is titled "84	th Ave. So. To So. Gra	dy Way".	The wa	ılls i
101490 1	<u> </u>	Ispecifie	d as stage 1 walls wer	e designed in stage 1 l	out were n	ot cons	tructed.
 Stage 2	•	Stage 2	shelf project is titled "	SR 167 - Main St. To 8	4th Ave. S	o. HC	V And
Joiage Z.	·	SCADI	This project was late	r modified and the walls	mentione	ed here	as -
<u> </u>	1	stage 2	shelf walls were elimi	nated from the project I	oefore it w	ent to A	.d.
}	<u> </u>	Jordan T	SHOP WORK WORK		,		

3/19/97

SR167 STAGE 3 WALL LIST

84TH AL3	(S/B on)	ace	L 884+60 (86.8' LT)	AL3' 89+60 (28.58' LT)	5	*	· 5
<u> </u>	Soils Repo	rt Sta	LM 892+50	AL3 89+30			
	PS&E Shel		L 884+60 (89.2' LT)	AL3' 89+88 (30' LT)			
	(stage 2)	ГСОРУ	L 804+00 (83.2 L1)		<u> </u>		
	(Stage 2)			1 .			
							
·		•		1			
84TH AR3	(N/B off)		L 877+90 (67.16' RT)	AR3' 83+70 (15' RT)	10		6
	Soils Repo	rt Sta.	None	None			
	PS&E Shel	f Copy	L 877+90 (68.15' RT)	AR3' 83+65.85 (16' RT)			
	(stage 2)		•				
			•				
180TH AL	2' (S/B on)		AL2' 54+14.04 (22.48' LT)	AL2' 55+72.02(22.14' LT) =	<i>1</i> √ 6		7
			L 1054+30.68(90.74' LT)	L 1055+88.63 (93.57, LT)			
	Stage 1 PS	S&E Copy	L 1052+12.21 (83.9' LT)	AL2' 55+90 =		1	
				L 1056+07.9 (94.3' LT)	1		
-			Soil Nail Wall was de	esigned in stage 1			
_							
180TH DR	2' (N/B on)	' "	L 1075+60 (69.8' RT)	L 1083+50.29 (54' RT)	7	<u> </u>	8
				•			
	Stage 1 PS	S&E Copy	L1075+60 (70.68' RT)	L 1083+50.29 (54.87' LT)			
			Wall was designed in	n stage 1 and no chang	e to the d	esign is	
			anticipated.				
					·		
NOTES:				4			
Soils Re	port:	Soils repo	ort was compiled in M	arch of 1992 under the	title "15th	n St S.W.	To
		South Gra	ady Way"				
Stage 1:		Stage 1 of	SR 167 is titled "84th	n Ave. So. To So. Grad	y Way". Л	he walls	
	}	specified a	as stage 1 walls were	designed in stage 1 bu	it were no	t constru	cted.
Stage 2:	1	Stage 2 sl	nelf project is titled "S	R 167 - Main St. To 841	th Ave. Sc	. HOV	And
	}	SC&DI. T	his project was later i	modified and the walls i	nentioned	l here as	
<u> </u>		stage 2 sh	nelf walls were elimina	ated from the project be	fore it we	nt to Ad.	
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Page 2

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F' 26.66-A	(Rev. 5-67)	. •							:
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WASHINGTON STATE HIGHWAY COMMISSION DEPARTMENT OF HIGHWAYS

Original	to Materials	Engineer
Copy to	Bridge Engin	ccr
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S.R. 5/6 Section	WASHINGTON STATE HIGHWAY COMMISSION DEPARTMENT OF HIGHWAYS LOG OF TEST BORING SR 167 NB, SB, Pamps Signal # 5	Original to Materials Engineer Copy to Bridge Engineer Copy to District Engineer Copy to Job No. 4-3/06 Cont. Sec. 35
SR 5/6 I/1+64	Signal # 5 Offset 55.0' 1, to	Ground El. <u></u>
or U.C.	9ers Casing Date 8-3-82	Sheet of 2
BLOWS PROFILE SAMPLE TUBE NOS.	O. C. DESCRIPTION OF MATE	ERIAL
	dry brown 5/19bJLY sandy	
1 U-1 abc	dry brown silty fine	
P-2 10 4 Rec 1.3	SAND wet brown sl silty SANI	
P-3 1 Rec 1, 4 1 7	clayey grey silt with some	organic material
1 U-2 abcdet	of silty grey clay from	AND with lenses
P-48 4 Rec 2,0		medium SAND
U-3 abcde	wet black as silty finet	·
P-5 6 Rec 1.	wet black al silty fine	to medium SAND
↓ ↓ ↓ b		

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	_ Sub Section_	Signal #5 Sheet 7 of 2	
BLOWS PROFILE	SAMPLE TURE NOS	DESCRIPTION OF MATERIAL	
P-6	13 6 Rec 20	wet black st silty fine to medium SAND	·
P-7		wet black s) sitty fine to medium SAND	
11	15 Rec 1.41		•
		note: washed @ - 30.0'	•
		Find of Boring -25.0'	
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iale Labo	ENGINEER				Place Seattle
Box 167,	Olympia, W	VA 98504 (1	Mailing Addre	ss) ·	Date 9-1-82
So. 2nd A	ve. shington 98:	504 (Shippi	ng Address)		
أ أ					PISTILBED
Dear Sir:					Samples
I have	forwarded	by today's	State G	1	. the following Foundation Samples.
ontract o	or <u></u>	3106	Section SR No.	516	Willis St Signals Sub-Section Signal 116.5
Station &		4, 55.0			Holic #
) sci		Depth	Tube Position in Sampler	Clas.	Description
Lab No.	Drive #	-0.5'	III Sampres	ML	LIE (163-2 VIT. CLZ, CALD SCORE Moist, fan, JK. brn. SILT
185	P-1	1	H20=12%	í	Layers of F. SAND & Rust Streaks
^ /		-5.0'		ML	in Lit is Lyening or Rust
رب م	P-2	to -6.3'	•	1116	
12	ļ <u>.</u>	-6.3		 	272 2255-1
_	P-3	-7.5'		Lur	ARONE mily Wings / willy
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py with samples lupy to addressee

Yours very truly, DIST_ MAT'LS_ENGR... 450 So. Spokane St.

BORING NO. __7__

Logged By DBG
Date __6-14-91_

ELEV. __60±

1							
Graph	US CS	Soil Description	Depth (ft.)	Sample	(N) Blows Ft.	W (%)	
	SM	Black, silty SAND, fine to medium grained, wet.	-70	H	29	24	
		wet, dense.	-75	Ţ	60	23	
	:		-80	工	68	*	
			-85	エ	59	*	,
	SP GP	Black SAND and GRAVEL, Wet, dense.	-90	I	64	26	
	GP		-95	エ	63	26	
			<u> </u>		46	15	

Boring completed at depth 99 feet. Groundwater noted at 27 feet.



Geotechnical Consultants

BORING LOG STATE ROUTE 167 KING COUNTY, WASHINGTON

Proj. No. 1630

Date 10-91

Figure A-8

BORING NO. 12

Logged By <u>DBG</u>
Date <u>6-24-91</u>

ELEV. 59±

Graph	US CS	Soil Description	Depth (ft.)	Sample	(N) Blows Ft.	(%)	
		Tan to gray, silty, gravelly SAND, fine to medium grained, moist to wet, dense. (FILL)	-5	エ	56	10	
	SM		10	エ	72	7	
		·	-15	エ	100+	10	
			20	工	44	8	
			-25	工	84	6	
		Gray to black, silty SAND, fine grained, moist, loose to medium dense.	30	工	13	32	
	SM SP	(Lenses of clean SAND)	-35	工	8	33	
	ML	Gray, fine sandy SILT with trace organics, wet, soft.	40	エ	4	35	-
		Becomes stiff.	45	工	15	46	
<u>-</u>	SM	Black silty SAND fine grained wet, loose	-50	エ	7	34	
	SP	(Some SILT lenses) Black SAND, wet, dense.	-55	I	40	25	;
			60	二	17	29	
			-65	工	59	21	

BORING CONTINUED ON NEXT PAGE



BORING LOG STATE ROUTE 167 KING COUNTY, WASHINGTON

10.04

Proj. No. 1630 Date 10-91 Figure A-13

BORING NO. 12

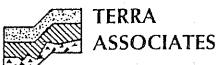
Logged By DBG

Date __6-24-91___

ELEV. <u>59±</u>

Graph	US CS	Soil Description	Depth (ft.)	Sample	(N) Blows Ft.	W (%)	
			-70	Н	25	*	
	SM	Black silty SAND with occ'l silt layers.	-75	エ	63	25	
			-80	工	65	23	
	SP	Becomes less silty.	-85	エ	74	*	
			-90	エ	90	29	~~ <u>~</u>
	SP GP	Black SAND and GRAVEL wet, dense.	-95	I	48	21	
	GP			T	93	13	

Boring completed at depth 99 feet. Groundwater noted at 32 feet.



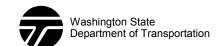
Geotechnical Consultants

BORING LOG STATE ROUTE 167 KING COUNTY, WASHINGTON

Proj. No. 1630

Date 10-31

Figure A-13

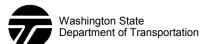


S.R. 167 SECTION Main Street to 84th Avenue	Job No.	L-1511	
Hole No. H-13-93 Sub Section Wall #10		Cont. Sec.	1766
Station W2 12+50	Offset 8.0' Rt. of Wall CL	Ground El.	47.0'
Type of Boring Skid Rig	Casing	W.T. El	34.0'
Inspector	Date September 16, 1993	Sheet 1	of <u>3</u>

- 1-					Date September 10, 1995 Sheet 1 Of	
DEPTH	BLOWS PER FT.	PROFILE	SAMI TUBE		DESCRIPTION OF MATERIAL	WEI PP
	81/11"		STD PEN 1	23 31 50/5"	Very dense, brown, moist, silty, fine to coarse sandy GRAVEL with occasional cobbles. Retained 1.0 ft.	
5	68		STD PEN 2	20 18 50	*SP-SM, M.C.=5.0% Very dense, brown, moist, silty, fine to coarse sandy GRAVEL with occasional cobbles. Retained 0.5 ft.	
10	10		STD PEN 3	4 5 5	ML, M.C.=27.5% Stiff, mottled dark brown, moist, fine sandy SILT with trace of organics. Retained 1.0 ft.	
15	3	x x x x x x x x x x x x x x x x x x x	STD PEN 4	1 2 1	Soft, mottled, wet, very sandy SILT with trace of organics. Retained 1.5 ft.	
20			U-1	A B C D	ML, M.C.=57.4% Gray, wet, fine sandy SILT with fine lenses of fibrous organic material.	

Continued Next Page

DOT FORM 351-003 REVISED 12/79

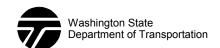


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S.R.	167	SECTION	Main Stree	t to 84th Avenu	e South		Job No.	<u>L-151</u>	11	_
Hole	No. <u>H-1</u>	13-93	Sub Section	Wall #10			Cont. Sec.	1766		_
Stati	on <u>W2 12</u>	2+50			Offset	8.0' Rt. of Wall CL	Ground El	47.0'		
Туре	of Boring	Skid Rig			Casing		W.T. El	34.	.0'	
Inspe	ector				Date	September 16, 1993	Sheet 2	_ of	3_	
PTH	BLOWS PER FT.	PROFILE	SAMPLE TUBE NOS.			DESCRIPTION OF MATERIAL	-		WELL PP	
	4	× · · × · ×	STD 1 PEN 1	ML, M.C.= Soft, wet,	fine sand	y SILT with fibrous organic r	naterial.			

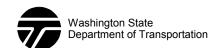
DEPTH	BLOWS PER FT.	PROFILE	SAMP TUBE N	LE IOS.	DESCRIPTION OF MATERIAL	WELL PP
	4	x · · x ·	STD PEN 5	1 1 3	ML, M.C.=39.0% Soft, wet, fine sandy SILT with fibrous organic material. Retained 1.5 ft.	
		×, ×, × ×, ×, × ×, ×, ×	U-2		No recovery.	
25	37		STD PEN 6	6 11 26	Dense, black, wet, silty, fine to medium SAND . Retained 1.0 ft.	
30	3	× × × × × × × × × × × × × × × × × × ×	STD PEN 7	2 1 2	ML, M.C.=41.4% Soft, gray, wet SILT. Retained 1.2 ft.	
		× · · × · × · × · × · × · × · × · × · ×	U-3		No recovery.	
35	26		STD PEN 8	10 12 14	Very stiff, gray, wet, fine sandy SILT . Retained 0.2 ft.	
40		× × × × × × × × × × × × × × × × × × ×				

Continued Next Page

DOT FORM 351-003 REVISED 12/79



S.R.	167	SECTIO	N <u>Main</u>	Street t	o 84th Avenu	e South			Job No		L-15	11
Hole	No. <u>H-1</u>	13-93	Sub Sec	ction <u>\</u>	Vall #10				Cont. S	ec.	1766	
Statio	on W2 12	2+50				Offset	8.0' Rt. of Wall Cl		Ground	EI.	47.0'	
Туре	of Boring	Skid Rig				Casing			W.T. E		34	.0'
Inspe	ector					Date	September 16, 19	93	Sheet	3	of	3_
DEPTH	BLOWS PER FT.	PROFILE	SAMF TUBE I				DESCRIPTION OF	MATERIAL				WELL PP
50	15	x	STD	1 3 12	End of tes *Laborato observed This is a s	wet, fine 1.5 ft. It hole book ry test rest in the fiel summary	sandy SILT with fill ring at 41.5 ft. below sults reflect the fine d. Log of Test Boring. sual field identificat	w ground er fraction	elevatior of soil	i.	ns	
55		_										
		_										
		_										
60												

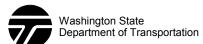


S.R. 167 SECTION Main Street to 84th Avenue	e South	Job No. <u>L-1511</u>
Hole No. H-14-93 Sub Section Wall #10		Cont. Sec. <u>1766</u>
Station _ W2 10+50	Offset 6.0' Rt. of Wall CL	Ground El. 45.0'
Type of Boring Skid Rig	Casing	W.T. El. 33.0'
Inspector	Date September 21, 1993	Sheet <u>1</u> of <u>3</u>

· ·					Date <u>September 21, 1995</u> Sheet <u>1</u> Of	
DEPTH	BLOWS PER FT.	PROFILE	SAMP TUBE N		DESCRIPTION OF MATERIAL	WEL PP
	56		STD PEN 1	13 26 30	ACP Very dense, brown, moist, silty, gravelly, fine to coarse SAND. Retained 1.0 ft.	
5	8		STD PEN 2	10 4 4	SP-SM, M.C.18.2% Loose, gray, moist, silty, gravelly, fine to coarse SAND. Retained 1.5 ft.	
10	13		STD PEN 3	7 6 7	SP, M.C.=4.9% Medium dense, brown, moist, slightly silty, fine to coarse SAND with root hairs. Retained 1.5 ft.	
15	3	X X X X X X X X X X X X X X X X X X X	STD PEN 4	1 1 2	ML, M.C.=41.7% Soft, gray, wet, fine sandy SILT with fibrous organic material.	
20		x x x x x x x x x x x x x x x x x x x	U-1	A B	Loose, gray, wet, silty, fine SAND . Retained 1.5 ft.	

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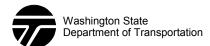


S.R.	167	SECTION	N <u>Main</u>	Street t	to 84th Avenu	e South			Job No		L-151	11
Hole	No. H-	14-93	Sub Se	ction _\	Wall #10				Cont. S	Sec.	1766	
Statio	on <u>W2 1</u>	0+50				Offset	6.0' Rt. of Wa	all CL	Ground	I EI.	45.0'	
Туре	of Boring	Skid Rig				Casing			W.T. E	l	33.	.0'
Inspe	ector					Date	September 2	1, 1993	Sheet	2	of	3_
DEPTH	BLOWS PER FT.	PROFILE		IPLE NOS.			DESCRIPTIC	N OF MATERIAL				WELL PP
	14	× · · × · × · × · × · × · × · × · × · ×	STD PEN 5	4 6 8	ML, M.C.= Stiff, gray, Retained	wet, fine	sandy SILT .					
		× · · × · · ×			Tried to pu	ush U.D.	at 22.5 ft., met	with resistanc	e.			
25	17		STD PEN 6	8 9 8	SP-SM, M. Medium d Retained	ense, dar		lty, fine to med	ium SAN	ND.		
30	11		STD PEN 7	1 5 6	Medium d medium S		k gray, water l	oearing, silty, f	ne to			
		× × × × × ×	U-2	B C	Medium st Retained	tiff, dark ç 1.0 ft.	gray, wet, fine	sandy SILT .				
35	28	× · × · × · × · × · × · × · × · × · × ·	STD PEN 8	10 15 13	ML, M.C.= Very stiff, Retained	dark gray	, wet, fine san	dy SILT .				

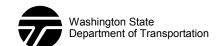
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S.R.	167	SECTION	۱ <u>Mair</u>	1 Street f	to 84th Avenue South	Job No.	<u>L-151</u>	1
Hole	No. <u>H-1</u>	4-93	Sub Se	ection _\	Wall #10	Cont. Sec.	1766	
Statio	on <u>W2 10</u>)+50			Offset 6.0' Rt. of Wall CL	Ground El.	45.0'	
Туре	of Boring	Skid Rig			Casing	W.T. El	33.	.0'
Inspe	ector				Date September 21, 1993	Sheet 3	of	3_
DEPTH	BLOWS PER FT.	PROFILE		MPLE E NOS.	DESCRIPTION OF MATERIAL			WELL PP
	34	× × × × × × × × × × × × × × × × × × ×	STD PEN 9	18 20 14	Hard, dark gray, wet, fine sandy SILT . Retained 1.5 ft.			
					End of test hole boring at 41.5 ft. below ground	elevation.		
		_			This is a summary Log of Test Boring. Soil/Roo are derived from visual field identifications and data.	k descriptior aboratory te	ns st	
45								
		-						
		-						
50								
55								
		-						
		-						

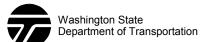


S.R. 167 SECTION Main Street to 84th Avenue	e South	Job No.	L-1511
Hole No. H-15-93 Sub Section Wall #10		Cont. Sec.	1766
Station D1 21+00	Offset 20.0' Lt. of Wall CL	Ground El.	45.0
Type of Boring Skid Rig	Casing	W.T. El	35.0'
Inspector	Date September 22, 1993	Sheet 1	of <u>2</u>

msp	ector				Date September 22, 1993 Sheet _1 of	
DEPTH	BLOWS PER FT.	PROFILE	SAMF TUBE N	PLE NOS.	DESCRIPTION OF MATERIAL	WELL PP
	34		STD PEN 1	13 17 17	ACP Dense, brown, moist, silty, fine to coarse sandy GRAVEL . Retained 0.8 ft.	
5 5	88		STD PEN 2	20 43 45	Very dense, brown, moist, dense, silty, fine to coarse sandy GRAVEL . Retained 0.8 ft.	
10	15		STD PEN 3	15 7 8	 ✓ Medium dense, gray, wet, silty, fine to coarse sandy GRAVEL. 	
15	7		STD PEN 4	3 4 3	Medium stiff, gray, wet, very sandy SILT .	
		x x x x x x x x x x x x x x x x x x x	U-1	A THRU F	Medium stiff, gray, wet, sandy SILT .	

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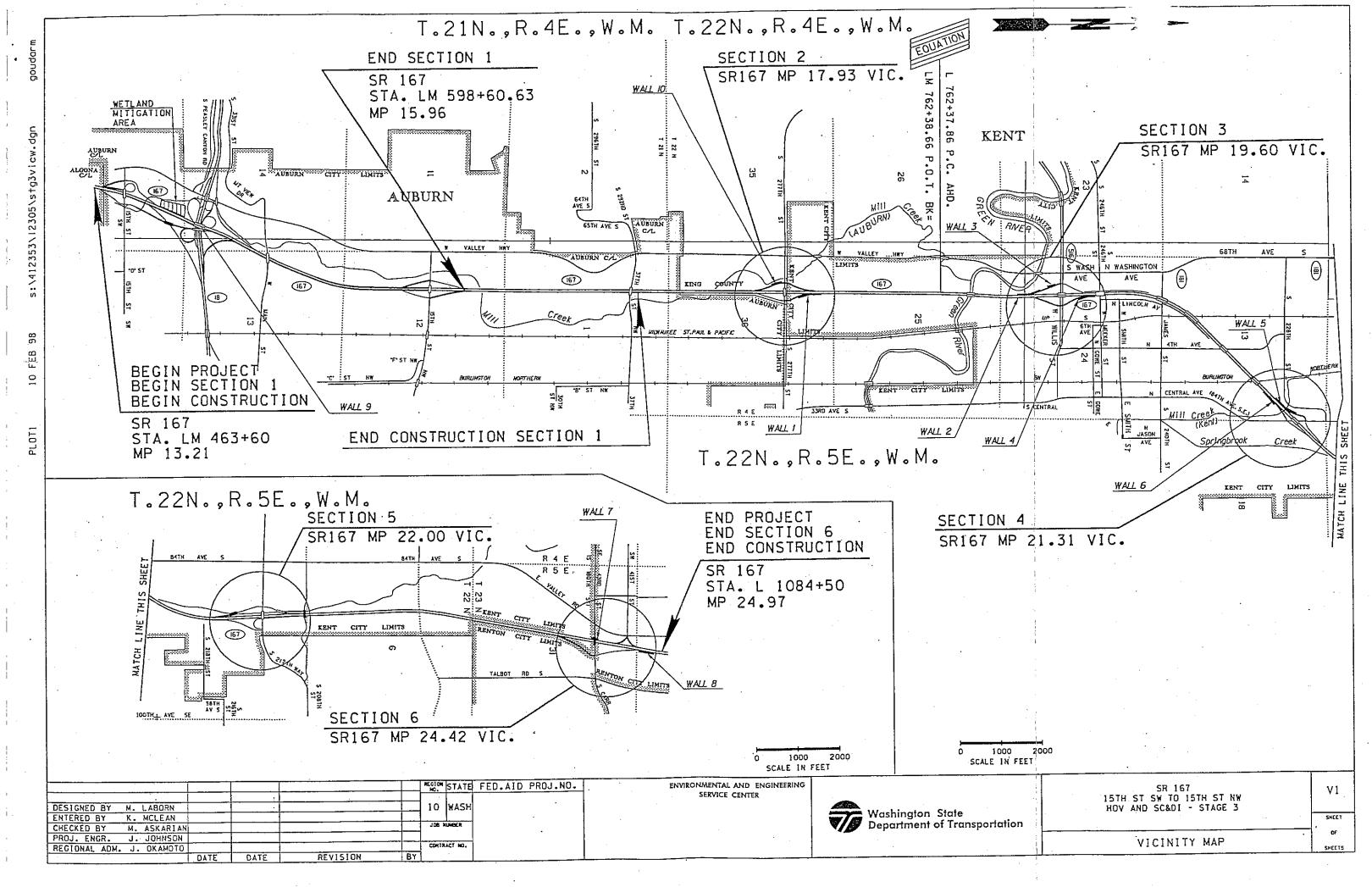
DOT FORM 351-003 REVISED 12/79



C D	407	CECTION	Main Cture	4 4 a 0 44 b Avramour	. Cauth		lah Na		1 454	4	
5.K.	167	SECTION	N <u>Iviain Stree</u>	t to 84th Avenue	e South		Job No	•	<u>L-151</u>	1	_
Hole	No. <u>H-1</u>	5-93	Sub Section	Wall #10			Cont. S	ec.	1766		_
Statio	on D1 21	+00			Offset	20.0' Lt. of Wall CL	Ground	EI.	45.0		_
Туре	of Boring	Skid Rig			Casing		W.T. El	·	35.	0'	_
Inspe	ector				Date	September 22, 1993	Sheet	2	of	2	
PTH	BLOWS PER FT.	PROFILE	SAMPLE TUBE NOS.			DESCRIPTION OF MATERIAL				WELL PP	ì
	15	×, ′, ×, ′	STD 5	ML. M.C.=	29.5%						i

DEPTH	BLOWS PER FT.	PROFILE	SAMI TUBE		DESCRIPTION OF MATERIAL	WELL PP
	15	x	STD PEN 5	5 7 8	ML, M.C.=29.5% Stiff, black, wet, fine sandy SILT. Retained 0.9 ft.	
25	5		STD PEN 6	3 2 3	Medium stiff, black, wet, fine sandy SILT .	
	25	,	STD PEN 7	7 8 17	SP, M.C.=19.7% Dense, black, wet, fine to coarse SAND. Retained 1.5 ft.	
30	26		STD PEN 8	8 9 17	Dense, black, wet, fine to coarse SAND . Retained 1.5 ft.	
		-			End of test hole boring at 31.5 ft. below ground elevation. This is a summary Log of Test Boring. Soil/Rock descriptions are derived from visual field identifications and laboratory test data.	
35		_				
		-				
40						

DOT FORM 351-003 REVISED 12/79 End of boring at 31.5 feet.



WALL SITE DATA CHECKLIST

Region NO SR 167 Job No. 01-2305 Date March 9, 1997 Project 1541- to 1571 HOV Ramp Bypasses - Stage 3				
Back Slope Slope Backfill Backfil				
Wall No. or Name 3 Begin Station WALL $D+DD$ End Station WALL $1+56$ Max Ht = $8'$ Max H = $5'$ S = 9 Back Slope = 9 H: V. Front Slope = 9 H: 9 V				
Wall No. or Name Begin Station End Station Max Ht = Max H = S = Back Slope = H: V Front Slope = H: V Region Project Office 412353 Region Contact Person Kenny Ezeo ke				
 ☑ Plans showing location of wall or reinforced slope: ☑ Utilities (existing or proposed) which may influence wall design/selection are shown ☑ Buildings, bridges, existing and proposed adjacent walls, culverts, drainage structures, and/or minor structures which may influence wall design are shown ☑ Right of way lines and other geometric constraints to wall construction are shown ☑ Existing ground contours are shown ☑ Wall/reinforced slope profiles: ☑ Existing grades in front of and behind the wall are shown 				
□ Final grades in front of and behind the wall are shown □ Wall/reinforced slope cross-sections: □ Provided at approximately every 15 m or at m 25ft □ Shows existing and proposed groundlines □ Shows locations of existing and proposed utilities (hove) □ Shows locations of culverts, drainage structures, and adjacent structures which may interfere with wall/reinforced slope or which may create surcharge loads □ Shows intensity and aerial extent of surcharge loads, if known				
☑ Concrete traffic barrier and/or beam guardrail locations provided as needed				
🖾 Desired aesthetics described				

WALL SITE DATA CHECKLIST

Region NW REGION	SR 167	Job No. OL 2305	Date 2/17	198
Region NIW REGIVIA	D.C. 1 D 1	<u> </u>	- LI LALUEC	C-1 C C 7
Project 15TH ST SW To) 15TH	ST NW, HU	JV LANES	->IAGE ->
Flujed 13111 - 1 974 - 1				

Back	Back Slope	
Slope S Backfill Ht Wall Fill Retained Soil Bearing Elev. Bearing Soil	S Backfill	E = 3'
Semi-Gravity Cantilever Wall	Gravity Wall	7167 75
Wall No. or Name $\#10$ Begin St Max Ht = 6.68 Max H = 3.68 S = 0	ation <u>ALI 11+50</u> End Station 2 Back Slope = H: V Front Sl	lope =H:V
Wall No. or Name Begin St Max Ht = Max H = S =		
Region Project Office JOHN JOHNSON	_ Region Contact Person KENI	NY EZEOKEKE
Plans showing location of wall or reinforced Utilities (existing or proposed) which may Buildings, bridges, existing and proposed and/or minor structures which may influx Right of way lines and other geometric of Existing ground contours are shown	ay influence wall design/selection a d adjacent walls, culverts, drainage ence wall design are shown	structures,
Wall/reinforced slope profiles: X Existing grades in front of and behind the X Final grades in front of and behind the v	ne wall are shown wall are shown	
Wall/reinforced slope cross-sections: ☐ Provided at approximately every 15 m of Shows existing and proposed groundling Shows locations of existing and proposed Shows locations of culverts, drainage strenterfere with wall/reinforced slope or wall Shows intensity and aerial extent of sur	es ed utilities ructures, and adjacent structures w which may create surcharge loads	hich may
Concrete traffic barrier and/or beam guardr	ail locations provided as needed	
☐ Desired aesthetics described		
_		

Special Provisions

Concrete Modular Retaining Wall

Description

This work consists of constructing a concrete interlocking block retaining wall in accordance with these Special Provisions and in close conformity to the lines, grades, and typical sections shown in the Plans or established by the Engineer.

Material

Individual blocks shall be constructed with LOCK-BLOCK molds and shall be $1.5 \times 0.75

Blocks shall have keys that will lock together when subsequent layers are turned either parallel or perpendicular to the wall face. The surface finish of the exposed face of the blocks shall be without large blemishes, such as honeycombed areas or chipped surfaces.

One supplier of LOCK-BLOCK molds is: Ultrablock, Inc., 7000 NE 40th Avenue, #D-3, Vancouver, WA 98661, 800-377-3877

Construction Requirements

Working Drawings

Prior to fabrication of the precast concrete blocks, the Contractor shall submit working drawings to the Engineer for approval. The drawings shall include, but not limited to, block dimensions, construction sequence and method, plan, and elevation of the wall.

Quality Assurance

The top of the wall shall be within two (2) inches of the line and grade shown in the plans.

Payment

The unit contract price per square foot for "Concrete Modular Retaining Wall" shall be to be the following full pay for performing the work as specified.